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Society for Rock Mechanics & Engineering Geology, Singapore Technical Seminar on Fiber Reinforced Sprayed Concrete State of the Art, Specifications, and Standards

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 Fibre reinforced sprayed concrete (FRS) is a mortar or concrete containing discontinuous steel or synthetic fibres, which are pneumatically projected at high velocity onto a surface



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Micro synthetic Fibres

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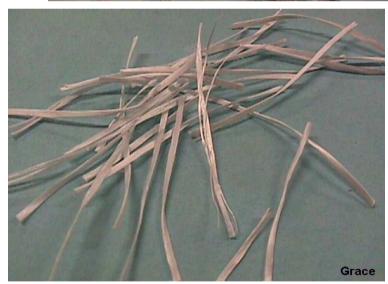
	multifilament	fibrillated		
		HJ HJ		
fibre section(s)				
# fibres (mio/kg)	200 - 300	10 - 20		
specific area (m²/kg)	200 - 300	40 - 80		

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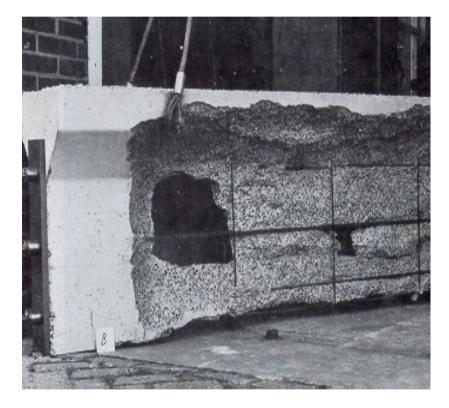


Macro synthetic Fibres

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- Modulus of Young reinforcing ability of the material
 - STEEL = 210 000 MPa
 - POLYPROPYLENE = 3000 5000 MPa
 - CONCRETE = 30 000 MPa
- Tensile strength reinforcing ability of the material
 - STEEL = 1000 2000 MPa
 - POLYPROPYLENE = 300-600 MPa
 - CONCRETE = 0 MPa
- Specific density
 - STEEL = 7850 kg/m³
 - POLYPROPYLENE = 910 kg/m³
 - CONCRETE = 2400 kg/m³
 - WATER = 1000 kg/m³
 - Contra: PP fibres float on water

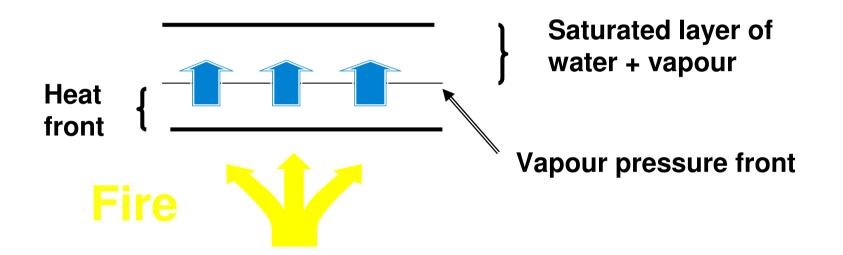
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Spalling of concrete

= When concrete is being exposed to fire, small or even big pieces are blown of





Exposed to fire, the water at the heat front evaporates \rightarrow this vapour will move towards the cold front and will be absorbed in the voids \rightarrow saturated layer \rightarrow when this layer cannot move fast enough, the heat front will reach this saturated front \rightarrow creation of higher vapour pressure will lead to spalling

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- How to avoid spalling of concrete?
 - adding monofilament PP fibres: these will melt (165 °C) and create high amount of 'channels' → the vapour pressure can escape more easily → no spalling
 - adding steel fibres: these will not prevent spalling but will hold the pieces together



	Micro Synthetic Fibre Reinforced Concrete	Steel Fibre Reinforced Concrete
strength and toughness		***
energy absorption		***
resistance to plastic shrinkage	***	
Freeze-thaw resistance	***	
resistance to spalling in a fire	***	*
impact resistance	*	***
resistance to drying shrinkage		***
structural reinforced concrete		***

Micro synthetic fibres = non-structural; Macro synthetic & steel fibres = structural

 \Rightarrow Micro synthetic fibres can never replace macro synthetic or steel fibres

 \Rightarrow However, they can be complementary

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- Length and diameter: aspect ratio (L/d)
- Shape of the fiber, or the anchorage
- Strength of the fiber
- Strength of the matrix (concrete)

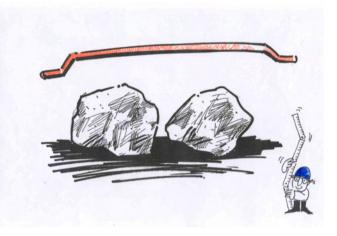




Steel fiber characteristics

Fiber length

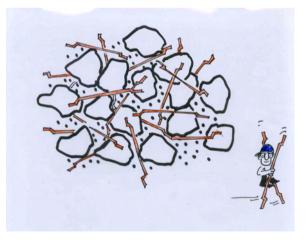
- 1. Bridge the crack
- 2. Overlap 2 aggregates
- 3. Workabilty : pumping, and hose diameter



Small fiber section

1.

- More fibers per kilo
- 2. Shorter distance between the fibers networking effect
- 3. More contact surface steel/ concrete per unit weight

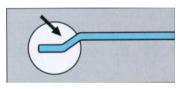


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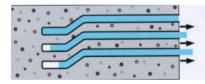
The quality of FRS is due to a combination of factors

A high length-diameter ratio (L/D ratio)

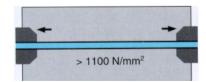
Hooked ends



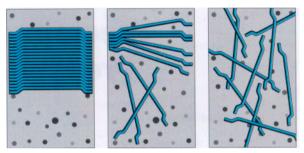
Controlled pull-out (due to deformation of the hook)

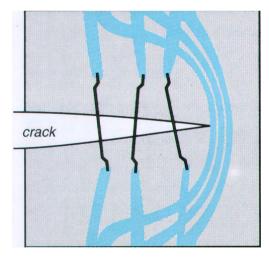


High tensile strength steel



A system of glued fibre bundles enables fibres with a high L/D ratio to be mixed easily and uniformly throughout the concrete







Concrete spalling is excluded

Spalling of concrete caused by rusting reinforcement bars is physically impossible with fibre reinforcement concrete due to :

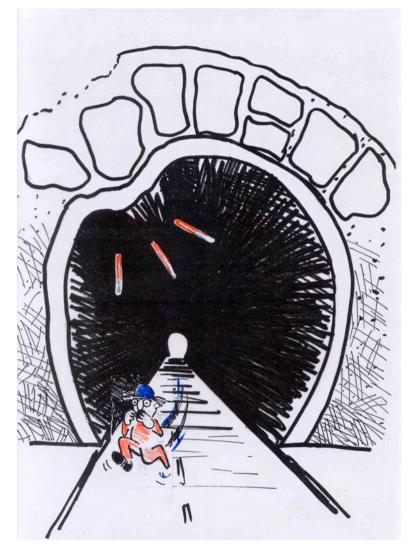
- small fibre diameter
- non continuous reinforcement
- high surface / volume ratio.



This type of damage is not possible with FRS



- Steel fibres are incorporated in the concrete to improve:
 - the ductility
 - the energy absorption
 - the crack resistance
 - the impact resistance
- Modern tunnelling methods include mobilising the strength of the ground by promoting ductility of the lining



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Sprayed Concrete

high initial strength for a good bond to the rock

ductility to absorb and block rock movement

safe working conditions

faster job progress

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Steel or macro synthetic fibres

provide a better bond due to a higher crack resistance

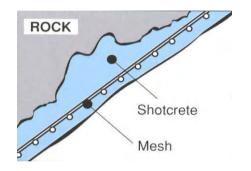
a high degree of ductility

incorporated reinforcement reducing the cycle time and increasing the safety for the tunnelling crew

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Mesh Reinforced Sprayed Concrete

Mesh installation is:



difficult

•time consuming

hazardous under loose rock

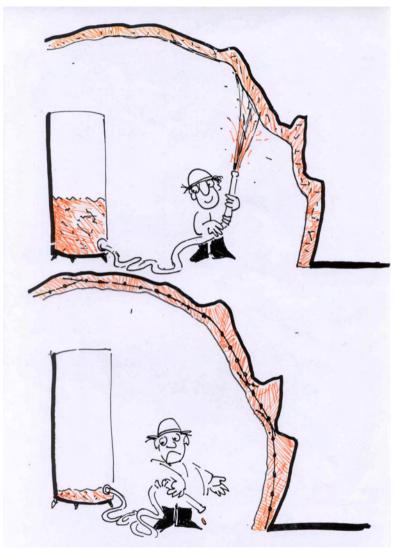
•costly

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Mesh Reinforced Sprayed Concrete

Sprayed concrete consumption is high due to:

- •irregular rock surface
- •filling up voids
- •high rebound on the mesh wires



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Mesh Reinforced Sprayed Concrete

Lining quality can be poor:

•Sprayed concrete builds up on the face of the mesh

•Low quality shadow areas form behind the wire

•Not a uniform bond between sprayed concrete and rock

•Irregular mesh positions do not give an efficient reinforcement

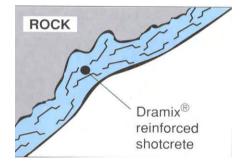




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Fiber Reinforced Sprayed Concrete

Advantages of a FRS layer:



Homogeneous reinforcement

•Can be applied very quickly

•A uniform thickness results in a significant reduction of sprayed concrete consumption

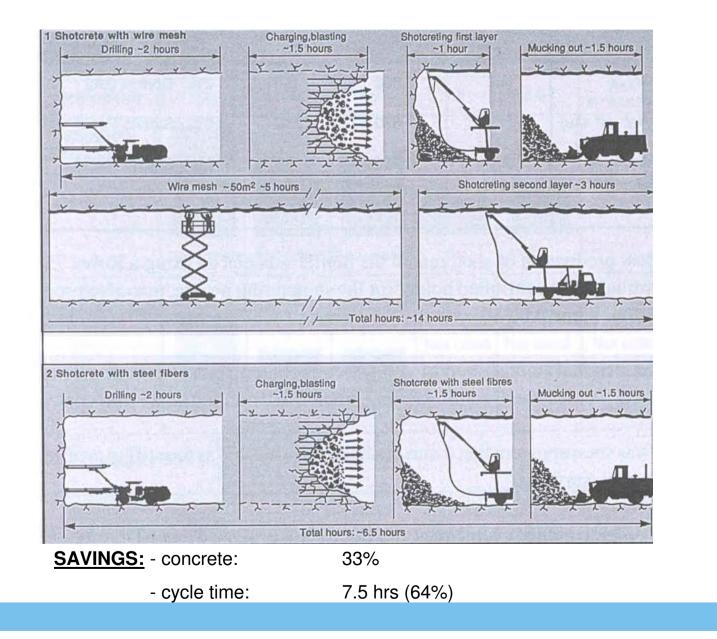
•Strong bond to the surface which is required to make the underground self supporting



- REBOUND

- Wet sprayed concrete (no silica fume): 15-20%
- Dry sprayed concrete: 20-50%
- Silica fume : improves bond
 - Rebound: 5-7% on vertical wall; 10-12% on ceiling
 - Typical dosage: 6-8% cement weight: makes the sprayed concrete not too strong
- Accelerator (AF)
 - High early strength
 - Improves the bond
 - Increases rebound if overdose



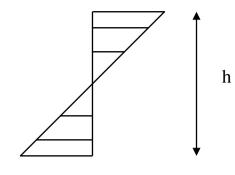


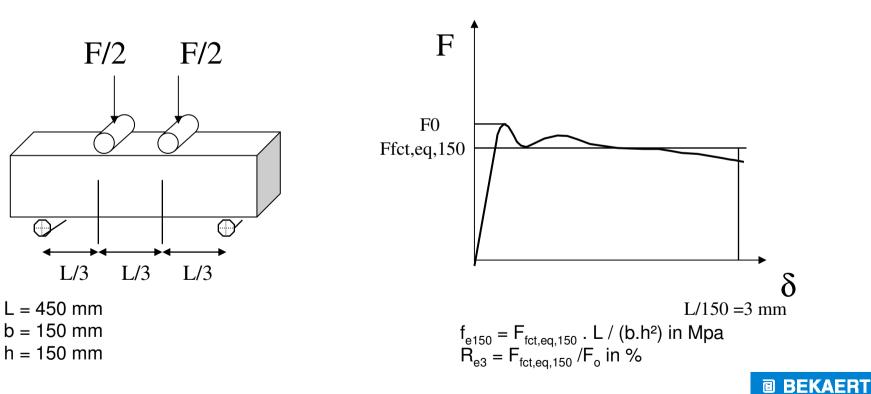
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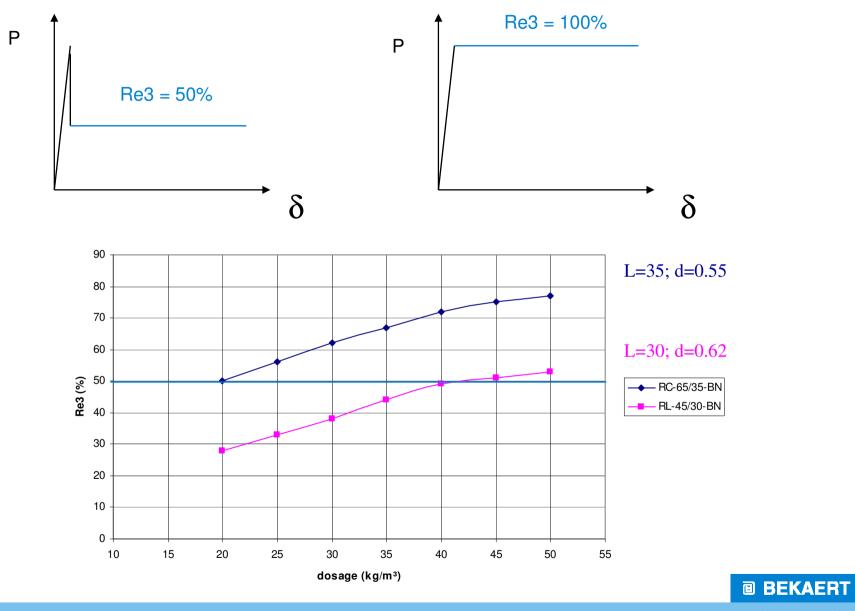
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Characterisation of FRS

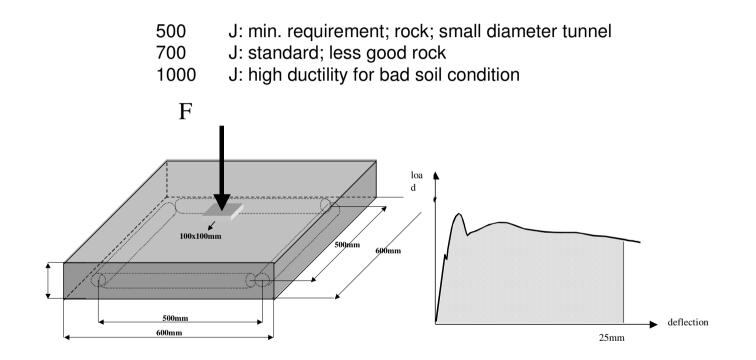
- 1. Beam test
- 2. Efnarc panel test





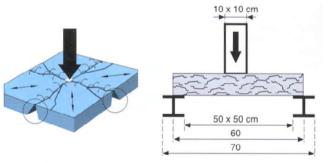


Efnarc toughness classification:





Efnarc panel test European standard EN 14488-5



The punching-flexion test is an ideal test to check the FRS behaviour:

- 1) A sprayed concrete tunnel ling behaves like a slab
- 2) The hyperstatic test conditions allow load redistribution
- 3) The test can be carried out with mesh reinforcement

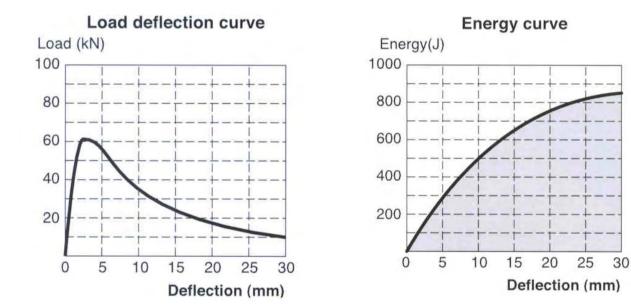
This test is introduced in 1989 by the French railway, and is in the meantime accepted by *Efnarc*, and is proposed in *CEN 104* Shotcrete.

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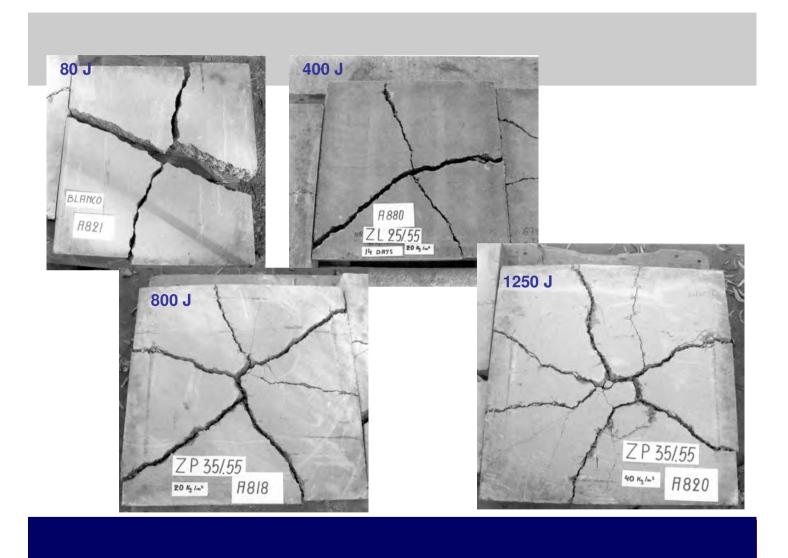


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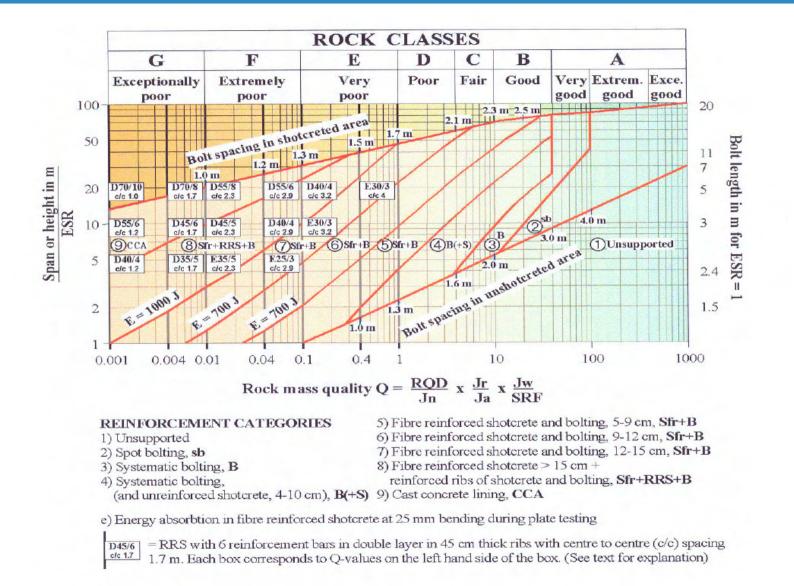
Efnarc panel test







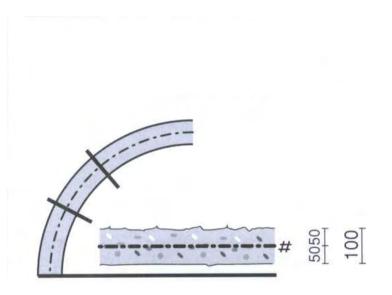
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1 mesh reinforced layer versus fibre reinforced sprayed concrete layer

Design of 1 mesh reinforced layer



$$M_{m,u} = 0.9 \cdot \frac{d}{2} \cdot A_s \cdot \sigma_{s,u}$$

whereby

- M_{m,u} = ultimate bending moment (Nm/m)
- d = total layer thickness (mm)
- $A_s = steel wire mesh cross secti (mm²/m)$
- $\sigma_{s,u}$ = ultimate tensile strength of me steel wires (N/mm²)

for
$$d = 100 \text{ mm}$$

$$A_{c} = 131 \text{ mm}^2 (\# 150/5)$$

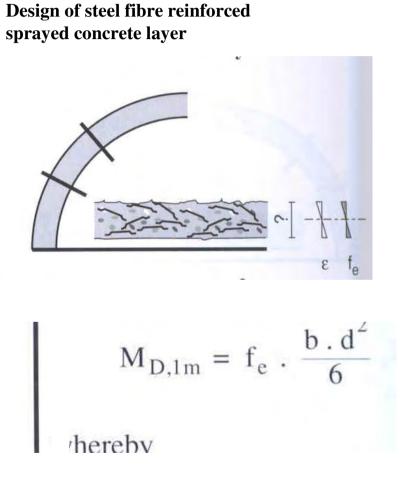
- $\sigma_{s.u} = 460 \text{ N/mm}^2$
- $M_{m,u}^{s,u} = 0.9 \text{ x } 50 \text{ mm x } 131 \text{ mm}^2/\text{n}$ 460 N/mm²
 - = 2.711.700 Nmm/m

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= 2.712 Nm/m

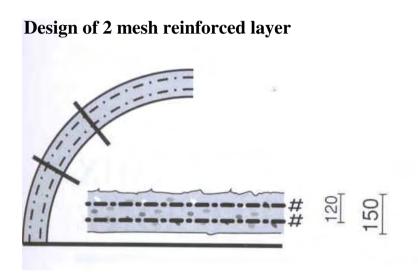
2



M _D	$M_{m,u} = M_{m,u}$ = bending moment (Nm/m)
0	
f _e	= equivalent flexural strength of
	Dramix shotcrete (N/mm ²)
b	= width of loaded area (mm)
d	= total layer thickness (mm)
for	$M_{D,1m} = 2.712 \text{ Nm/m}$
	$b^{D,1m} = 1.000 \text{ mm}$
	$f_e = 0.65 f_u \text{ for } 40 \text{ kg/m}^3 \text{ Dramix}$
	$T_{e} = 0.03 T_{u}$ 101 10 kg/m Dramk ZP 30/.50 (Fig. 21)
	$f_u = 4 \text{ N/mm}^2$
	$f_e = 0.65 \text{ x} 4 = 2.6 \text{ N/mm}^2$
d^2	$= \frac{6 \cdot M_{D,1m}}{f_{e} \cdot b} = \frac{6 \times 2.712.000}{2.6 \times 1.000}$
u	$= \frac{f_{e} \cdot b}{f_{e} \cdot b} = \frac{1}{2.6 \times 1.000}$
	$= 6.258 \text{ mm}^2$
	d = 80 mm

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Double mesh reinforced layer versus fibre reinforced sprayed concrete layer



$$M_{2m,u} = 0.9 \cdot h \cdot A_s \cdot \sigma_{s,u}$$

 $M_{2m,u}$ = ultimate bending moment (Nm/m) d - a (mm) 1 = = shotcrete layer 1 thickness (mm) = shotcrete cover of 1 wire mesh (mm) steel wire mesh cross = section (mm^2/m) ultimate tensile Σ_{s,u} = strength of steel mesh wires (N/mm²) 150 mm 1 = 30 mm 1 = 150 - 30 = 120 mm= $126 \text{ mm}^2/\text{m}$ (# 100/4) 460 N/mm² 5 s.u $M_{2m,u} =$ 0.9 x 120 mm x 126 mm²/m x 460 N/mm² 6.259.680 Nmm/m 6.260 Nm/m \equiv

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Double mesh reinforced layer versus fibre reinforced sprayed concrete layer

Design of steel fibre reinforced sprayed concrete layer

$$M_{D,2m} = f_e \cdot b \cdot \frac{d^2}{6}$$

$$M_{D,2m} = 6.260 \text{ Nm/m}$$

$$b = 1.000 \text{ mm}$$

$$f_e = 0.65 \text{ f}_u \text{ for } 40 \text{ kg/m}^3 \text{ Dramix}$$

$$ZP 30/.50$$

(Fig. 21)

$$f_u = 4 \text{ N/mm}^2$$

$$f_e = 0.65 \text{ x } 4 = 2.6 \text{ N/mm}^2$$

$$^2 = \frac{6 \cdot M_{D,2m}}{f_e \cdot b} = \frac{6 \text{ x } 6.260.000}{2.6 \text{ x } 1.000}$$

$$= 14.446 \text{ mm}^2$$

$$d = 120 \text{ mm}$$

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- EN14488-3 Testing sprayed concrete Part 3: Flexural strengths (first peak, ultimate and residual) of fibre reinforced beam specimens
- EN14488-5 Testing sprayed concrete Part 5: Determination of energy absorption capacity of fibre reinforced slab specimens
- EN14488-7 Testing sprayed concrete Part 7: Fibre content of fibre reinforced concrete





FIBRE COUNTER









- 1. Plug into A/C power point. Switch on the fibre counter.
- 2. Determine an amount of Steel Fibre Reinforced Sprayed Concrete (SFRS) in accordance to EN14488-7. Put the SFRS into the top of the fibre counter.
- 3. The SFRS will be vibrated as it passes through the fibre counter. The steel fibres will be stuck to the magnet in the fibre counter while the aggregates, sand and cement will appear at the base of the fibre counter.
- 4. Remove the steel fibres from the magnet. Clean the steel fibres with clean water. Dry the steel fibres. Weigh the amount of steel fibres collected. All other procedures will be in accordance to EN14488-7.
- 5. Clean the fibre counter with clean water after each use.

Time			Wa	11			Тор			Actual steel fibre dosage in mix, kg/m3	Density of plain shotcrete, kg/m3	Density of SFRS, kg/m3
12:00 PM	Shotcrete sample, kg	2.08	2.02	3.32		3.1	2.4	2.02	1.02	30	2200	2230
	Steel fibre weight, g	19.5	24.44	30.58		33.63	23.6	23.5	12.27		2200	2250
	Steel fibre dosage, kg/m3	20.90625	26.98	20.54		24.19	21.93	25.94	26.83			
	Average steel fibre dosage, kg/m3	22.8	090742	7		24.7222353						
	Steel fibre rebound, %	23.9	697524	4			17.59258825					
	Energy Absorption, Joules	^	>1000		>700			Tested in NTU on 14th August 2007				
	Reqd. Energy Absorption,											
	Joules	;	>700				>7	700				
	Designed steel fibre dosage, kg/m3		20			20						
4:00 PM	Shotcrete sample, kg	2.26	2.44	2.5	3.08	2.54		2.2		40	2190	2230
	Steel fibre weight, g	34.27	30.8	31.22	33.7	35.35	32	22.35	29.79	40	2190	2230
	Steel fibre dosage, kg/m3	33.81509	28.15	27.85	24.4	31.04	31.86	22.65	25.95			
	Average steel fibre dosage, kg/m3	2	28.55304604		27.87435708							
	Steel fibre rebound, %	2	28.6173	8491	491		30.31	41073				
			>1000 (not tested since the ceiling panel achieved the									
		ceiling p										
	Energy Absorption, Joules	re	equiren	nents)	ts)		1300			Tested in NTU on 14th August 2007		
	Reqd. Energy Absorption,	rgy Absorption,										
	Joules		>100	00		>1000						
	Designed steel fibre dosage, kg/m3	30		30								
Ba	sed on the above-mentioned resu	ults, the sp	oraying	trials	conduc	ted on	16th J	uly 200	7 were	successfully completed. The steel fibr	re rebound were within the desig	ned tolerances.

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Fibre Reinforced Sprayed Concrete Specifications

- Minimum fiber length: 2 times the max. Aggregate size
- Aspect ratio: range 40-80
- Fibre tensile strength: min. 1000 Mpa
- Ductility performance: 500-700-1000 Joules
- EN14487-1 Sprayed concrete Part 1: Definitions, specifications and conformity
- EN14487-2 Sprayed concrete Part 2: Execution

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